EXPRESS TERMS
FOR PROPOSED BUILDING STANDARDS
OF THE
OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT

REGARDING PROPOSED CHANGES TO
CALIFORNIA BUILDING CODE
CALIFORNIA CODE OF REGULATIONS, TITLE 24, PART 1, CHAPTERS 6 & 7

LEGEND FOR EXPRESS TERMS
1. Existing California amendments or code language being modified: All such language appears in *italics*, modified language is <u>underlined</u>.
2. New California amendments: All such language appears *<u>underline and in italics</u>*.
3. Repealed text: All such language appears in ~~strikeout~~.

EXPRESS TERMS:

CHAPTER 6. SEISMIC EVALUATION PROCEDURES FOR HOSPITAL BUILDINGS

ARTICLE 1. DEFINITIONS AND REQUIREMENTS

1.0 Scope. The regulations in this article shall apply to the administrative procedures necessary to implement the seismic retrofit requirements of the Alfred E. Alquist Hospital Facilities Seismic Safety Act of 1983.
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1.2 Definitions. Unless otherwise stated, the words and phrases defined in this section shall have the meaning stated therein throughout Chapter 6, Part 1, Title 24.

~~Alternate~~ <u>Alternative</u> Analysis means a complete seismic analysis using methodology approved in advance by the Office and meeting the criteria of Article 2, Section 2.7 of these regulations.
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1.4.5.1 Change in Seismic Performance Category. The SPC or NPC for a hospital building may be changed by the Office from the initial determination in Sections 1.3.3 or 1.3.4 provided the building has been modified to comply with the requirements of Chapter <u>34A</u> ~~16B~~, Part 2 of Title 24 for the specified SPC or NPC.

1.4.5.1.1 – The SPC or NPC for a hospital building may be changed by the Office from the initial determination made per Sections 2.0.1.2.3 or 11.0.1.2.1 upon the following:

A Seismic Evaluation Report shall be submitted and approved which shall include either or both of the following:

1.1 A structural evaluation report in accordance with Section 1.3.3;

1.2 A nonstructural evaluation report in accordance with Section 1.3.4.

Exception: To change an NPC 1 hospital building to an NPC 2 under this section, the nonstructural evaluation may be limited in scope to the systems and equipment specified in Section 11.2.1.

2. The building has been modified to comply with the requirements of Chapter ~~16B~~ <u>34A</u>, Part 2 of Title 24 for the specified SPC or NPC.

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1.5.2 Delay in Compliance

1. The Office may grant the hospital owner an extension to the January 1, 2008 seismic compliance deadline for both structural and nonstructural requirements if compliance will result in diminished health care capacity which cannot be provided by other general acute care hospitals within a reasonable proximity.

1.1 Hospital owners requesting an extension in accordance with Section 1.5.2 must submit an application form to the Office by January 1, 2007. The application form shall be accompanied by a statement explaining why the hospital is seeking the extension to the January 1, 2008 seismic compliance deadline. The statement shall include, at a minimum, the following information:

(a) The length/duration of the extension request;

(b) The hospital buildings requiring an extension; and

(c) The acute care services that will be completely or partially unavailable if the extension is denied.

1.2 The hospital owner shall request an extension for seismic compliance in one year increments, up to a maximum of five (5) years, beyond the mandated year of compliance. The hospital owner shall also submit an amended compliance plan and schedule in accordance with Section 1.4.5 indicating when compliance will be obtained.

2. Any general acute care hospital located in Seismic Zone 3, as defined by Section ~~1627A.2~~ 1627B.2 of the 1998 California Building Code may request an exemption from the anchorage and bracing requirements of NPC 3 if all the following conditions are met:

2.1 The hospital must meet the anchorage and bracing requirements for NPC 2 by January 1, 2002;

2.2 The hospital shall submit a site-specific engineering geologic report, prepared in accordance with Section 1634B.1 of the 1998 California Building Code. The report shall include estimates of the effective peak ground acceleration (EPA) with a 10 percent probability of exceedance in 50 years;

2.3 The California Geological Survey (CGS) ~~Division of Mines and Geology (CDMG)~~ reviews and approves the findings of the site-specific engineering geologic report;

2.4 The site-specific engineering geologic report demonstrates that the estimated EPA with a 10% probability of exceedance in 50 years is less than 0.25 g.

2.5 The hospital owner requesting the exemption shall pay the actual costs of OSHPD and CGS ~~CDMG~~ for the review and approval of the site-specific engineering geologic report.

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ARTICLE 2. PROCEDURES FOR STRUCTURAL EVALUATION OF BUILDINGS

2.0 General

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2.1 Site Visit, Evaluation, And Data Collection Procedures

2.1.1 Site Visit And Evaluation

1. The evaluator shall visit the building to observe and record the type, nature, and physical condition of the structure.

2. The evaluator shall review an Engineering Geological Report on site geologic and seismic conditions. The report shall be prepared in accordance with Title 24, Section 1634A of 1995 California Building Code (CBC) or equivalent provision in later version of the CBC.

Exceptions:

1. Reports are not required for one-story, wood-frame and light steel-frame buildings of Type II or Type V construction and 4,000 square feet or less in floor area;
2. A previous report for a specific site may be resubmitted, provided that a reevaluation is made and the report is found by the Office to be currently appropriate.

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2.4.9 Element Capacities. Calculate element capacities on the ultimate-strength basis of the 1994 NEHRP Recommended Provisions.

When calculating capacities of deteriorated or damaged elements, the evaluator shall make appropriate reductions in the material strength, the section properties and any other aspects of the capacity affected by the deterioration.

2.4.9.2 Steel. The basic document is Chapter 5 ~~6~~ of the 1994 NEHRP Recommended Provisions, as modified in Articles 4 and 6 of these regulations.

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2.4.9.4 Masonry. The basic document is Chapter 8 ~~12~~ of the 1994 NEHRP Recommended Provisions, as modified in Article 5 of these regulations.

2.4.10 Dynamic Analysis. Unless otherwise noted, the procedures given in Articles 3 through 10 use the equivalent lateral force procedure. The use of a dynamic analysis procedure is required for the following:

1) Buildings 240 feet or more in height;

2) Buildings with vertical irregularities caused by significant mass or geometric irregularities;

3) Buildings where the distribution of the lateral forces departs from that assumed in the equivalent lateral force procedure; and

4) Where required by the evaluation statements in Articles 3 through 10.

Dynamic analysis procedures shall conform to the criteria established in this section. The analysis shall be based on an appropriate ground motion representation as specified in this section and shall be performed using accepted principles of dynamics. Structures that are evaluated in accordance with this section shall comply with all other applicable requirements.

2.4.10.1 Ground Motion. The ground motion representation shall be an elastic response spectra developed for mean values for the specific site, in accordance with the procedures in Title 24, Section 1629A.2 of 1995 California Building Code (CBC) or equivalent provision in later version of the CBC.

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2.7 Alternative Analysis. The owner of a building may elect to perform an Alternative Analysis, to evaluate a structure in more detail than that provided by the evaluation procedures specified in these regulations. The methodology of an Alternative Analysis must be approved in advance by OSHPD, and shall meet the following criteria:

1. Data collection on the structure and site conditions shall be performed in accordance with the appropriate Sections of Article 2 of these regulations. Depending upon the type of analysis to be performed, additional data regarding the as built condition and material properties may be required;

2. The Alternative Analysis shall be based on a site specific ground motion as specified in Section ~~2.4.10.1~~ 3413A.1.2 of the 2007 California Building Code (CBC);

3. The analysis of the structure shall determine the distribution of strength and deformation demands produced by the design ground shaking and other seismic hazards. The analysis shall address seismic demands and capacities to resist these demands for all elements in the structure that either:

   - Are essential to the lateral stability of the structure (primary elements); or
   - Are essential to the vertical load carrying integrity of the building.

4. The analysis procedure may consist of a linear or nonlinear analysis. The analytical methods and acceptance criteria shall conform to Section 3403A.2.3.4 of the 2007 CBC and nonlinear time history analysis procedure shall be reviewed and approved, in advance, by OSHPD.

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ARTICLE 4. PROCEDURES FOR MOMENT RESISTING SYSTEMS

4.0 Introduction. Moment frames develop their resistance to lateral forces through the flexural strength and continuity of beam and column elements. Moment frames may be classified as special, intermediate, and ordinary frames.

For evaluations using these regulations, it is not necessary to determine the type of frame in the building. The issues are addressed by appropriate acceptance criteria in the specified procedures. For determination of element capacities, see Article 2, Section 2.4.9.

4.1 Frames With Infill Walls

4.1.1 Interfering Walls: All infill walls placed in moment frames are isolated from structural elements. For conforming buildings, the evaluator may consider this condition as mitigated, and no calculations are necessary. The deficiency is an inappropriate connection of the wall to the frame. Evaluate the relative strength and stiffness of the walls and frames, considering the nature and size of the joint or connection between the wall and the frame. If the strength of the walls is not commensurate with the stiffness, the building should be treated as Type 7 or Type 10 (Article 2, Section 2.2.3 “Common Building Types), a frame with infill walls. If the infill walls do not extend the full story height and are not properly isolated from the frame columns, evaluate the column shear demand and capacity, based on a column height equal to the clear distance from the top of wall to the bottom of the slab or beam above, amplifying the design forces in the short column by $C_d/2$, but not less than 1.5. The shear demand need not exceed the shear capacity corresponding to ~~the~~ flexural capacity of the column, based on a column height equal to the clear distance from the top of wall to the bottom of the slab or beam above.

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4.2.10 Pre-Northridge Earthquake Welded Moment Frame Joints. Welded steel moment frame beam-column joints are designed and constructed in accordance with recommendations in FEMA 267, Interim Guidelines: Evaluation, Repair, Modification and Design of Welded Steel Moment Frame Structures, August 1995.

For buildings constructed under permit issued after October 25, 1994, the evaluator may consider this condition as mitigated. The deficiency is in the ductility of the beam-column joint. The following procedures shall be used for categorizing buildings with welded steel moment frame joints:

Procedure for conforming buildings: Conforming buildings located in Seismic Zone 4 of 1995 California Building Code (CBC) or later version of the CBC, within a zone designated as being potentially subject to near field effects in strong ground shaking, shall be placed in SPC 3. All other conforming buildings shall be placed in SPC 4.

Procedure for nonconforming buildings: Nonconforming buildings shall be placed in SPC 2.
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ARTICLE 6. PROCEDURES FOR BRACED FRAMES

6.0 Introduction. Braced frames develop their resistance to lateral forces by the bracing action of diagonal members. The braces induce forces in the associated beams and columns so that all work together like a truss with all members subjected to stresses that are primarily axial.

A **Concentrically Braced Frame** has minor eccentricities in the joints of the frame that are accounted for in the design.

An **Eccentrically Braced Frame** has elements that are strictly controlled to combine a stiffening effect due to the diagonal braces with yielding in the link beams. Eccentrically braced frames are present only in conforming buildings.

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6.1.6 Connection Strength. All the brace connections are able to develop the yield capacity of the diagonals.

The deficiency is in the strength of the connections. Check the connection strength. Use a demand value that develops the tensile capacity of the brace or is 1.25 times the required seismic force. If connections in a conforming buildings cannot develop the yield capacity of the brace and do not meet the requirements of Part 2, Title 24, Section 2211A.9.3 of 1995 California Building Code (CBC) or equivalent provision in later version of the CBC, the building shall be placed in SPC 4.

6.1.7 Column Splices. All column splice details of the braced frames can develop the column yield capacity.

The deficiency is in the strength of the splice. Calculate the adequacy of the splice connection for all expected forces including gravity loads. Amplify the seismic load for partial penetration welded splices by the factor $C_d/2$ when the seismic load produces tension at the splice. If the column splice details in a conforming buildings cannot develop the yield capacity of the column and do not meet the requirements of Part 2, Title 24, Section 2211A.9.5 of 1995 California Building Code (CBC) or equivalent provision in later version of the CBC, the building shall be placed in SPC 4.

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ARTICLE 11. EVALUATION OF CRITICAL NONSTRUCTURAL COMPONENTS AND SYSTEMS

11.0 Introduction

This article covers nonstructural components and systems critical to patient care.

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11.2.2 Evaluation Procedures for NPC 3 and NPC 3R. The following steps shall determine if the building meets the criteria for NPC 3 or NPC 3R:

a) Identify the specific nonstructural components and equipment that are subject to the requirements of NPC 2 and NPC 3 or NPC 3R;

b) Conduct an inventory of components and equipment specified in Table 11.1, NPC 3 and NPC 3R, noting whether the components and equipment are anchored or braced;

> **Exception:** Any general acute care hospital facility located in both a "rural area" as defined in Section 70059.1, Division 5, Title 22 and Seismic Zone 3 pursuant to 1995 California Building Code (CBC) or later version of the CBC shall comply with the fire sprinkler system anchorage and bracing requirements of NFPA 13, 1994 edition or subsequent standard by January 1, 2013.

c) Determine the level of NPC 3 conformance desired.

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11.3 Testing Requirements For Evaluating The Performance Of Existing Mechanical Fasteners. A testing program shall be instituted to determine the capacity of mechanical fasteners used to anchor non-structural components including the bracing of pipes, ducts, and conduit, and the attachment of equipment and other components listed in the 1995 CBC, Part 2, Title 24, Table 16A-O. Anchors shall be categorized as either seismic bracing of pipes ducts or conduit or equipment and other component anchors.

11.3.1 Anchors Used in the Seismic Bracing of Pipes, Ducts, or Conduit. For anchors used in the seismic bracing of pipes, ducts, or conduit, the following shall apply:

1. 20% of the anchors (20 minimum) of a given size and type (wedge, shell and sleeve for expansion bolts), at each level of the structure shall be tension tested to 3 times the maximum calculated design load specified in Section 1630B of 1998 California Building Code (CBC) or equivalent provision in later version of the CBC but not less than 500 pounds. A minimum of one anchor in any 4-bolt group shall be tested assuming an equal distribution of the calculated force to the bolt group. One-quarter (1/4) inch diameter anchors need not be tested. Where none of the anchors in the group have calculated tension, testing shall consist of torque testing.

   Exception: Internally threaded anchors, such as shell type anchors, shall be tested to 4 times the maximum calculated design loads. Attachment hardware shall be shimmed or removed prior to testing so that it does not prevent the possible withdrawal of the anchor.

2. If an anchor fails the tension test, 20 anchors, installed by the same trade, in the immediate vicinity of the failed anchor shall be tested prior to resuming to a 20% sampling rate for testing.

11.3.2 Anchors Used in the Attachment of Equipment and Other Components. For anchors used in the attachment of equipment and other components listed in the 1995 CBC, Part 2, Title 24, Table 16A-O, The following shall apply:

1. A minimum of one anchor of a given size shall be tension tested for each piece of equipment or other component under consideration. Where the number of anchors for the piece of equipment or component exceeds four, a minimum of 20% of the anchors shall be tension tested. Where none of the anchors in the group have calculated tension, testing shall consist of torque testing.

2. The tension test load shall be 3 times the maximum tension force calculated for an anchor in the attachment group using the design loads specified in Section 1630B of 1998 California Building Code (CBC) or equivalent provision in later version of the CBC or 500 pounds minimum. One-quarter (1/4) inch diameter anchors need not be tested.

   Exception: Internally threaded anchors, such as shell type anchors, shall be tested to 4 times the maximum calculated design loads. Attachment hardware shall be shimmed or removed prior to testing so that it does not prevent the possible withdrawal of the anchor.

3. If a single anchor fails, all anchors in the attachment group shall be tested. If two (2) or more anchors fail, the component shall be retrofitted for the forces as for new construction.

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11.3.5 Allowable shear loads. Allowable shear loads on anchors shall be determined by either of the following:

1. Shear values listed in Table 19B-E of 1998 California Building Code (CBC) or equivalent provision in later version of the CBC, or;

2. Shear values shall be obtained by analysis using **Strength Design of Anchorage to Concrete,** Section A.6 published by the Portland Cement Association, 1999, with the specified reduction coefficient(s) to convert the “strength” values to allowable stress design values of 1.7.

Table 11.1 – NONSTRUCTURAL PERFORMANCE CATEGORIES

Time frames	Nonstructural Performance Category[1]	Description
	NPC 1	Buildings with equipment and systems not meeting the bracing and anchorage requirements of any other NPC.

January 1, 2002	**NPC 2**	The following are braced or anchored in accordance with Part 2, Title 24[1]: • communications systems, • emergency power supply, • bulk medical gas systems, • fire alarm systems; and • emergency lighting equipment and signs in the means of egress.
January 1, 2008	**NPC 3 / NPC-3R**	The building meets the criteria for NPC "2" and in critical care areas, clinical laboratory service spaces, pharmaceutical service spaces, radiological service spaces, and central and sterile supply areas, the following components meet the bracing and anchorage requirements of Part 2, Title 24[2]: • Nonstructural components, listed in the 1995 CBC, Part 2, Title 24, Table 16A-O, Part 2. **Exception:** For NPC-3R, lateral bracing of suspended ceiling systems may be omitted in rooms with a floor area less than 300 square feet, provided the room is not an intensive care or coronary care unit patient room, angiography laboratory, cardiac catheterization laboratory, delivery room, operating room, or post-operative recovery room. • Equipment, as listed in the 1995 CBC, Part 2, Title 24, Table 16A-O, "equipment" including equipment in the physical plant that service these areas. **Exceptions:** 1. Seismic restraints need not be provided for cable trays, conduit and HVAC ducting. Seismic restraints may be omitted from piping systems, provided that an approved method of preventing release of the contents of the piping system in the event of a break is provided. 2. Only elevator(s) selected to provide service to patient, surgical, obstetrical, and ground floors during interruption of normal power need meet the structural requirements of Part 2, Title 24[1]. • Fire sprinkler systems comply with the bracing and anchorage requirements of NFPA 13, 1994 edition or subsequent applicable standards. **Exception:** Acute care hospital facilities in both a rural area as defined by Section 70059.1, Division 5 of Title 22 and Seismic Zone 3 shall comply with the bracing and anchorage requirements of NFPA 13, 1994 edition or subsequent applicable standards by January 1, 2013.
	NPC 4	The building meets the criteria for NPC "3" and all architectural, mechanical, electrical systems, components and equipment, and hospital equipment meet the bracing and anchorage requirements of Part 2, Title 24 [2]. This category is for classification purposes of the Office of Emergency Services.
January 1, 2030	**NPC 5**	The building meets the criteria for NPC "4" and on-site supplies of water and holding tanks for wastewater, sufficient for 72 hours emergency operations, are integrated into the building plumbing systems. As an alternative, hook-ups to allow for the use of transportable sources of water and sanitary waste water disposal have been provided. An on-site emergency system as defined within Part 3, Title 24 is incorporated into the building electrical system for critical care areas. Additionally, the system shall provide for radiological service and an onsite fuel supply for 72 hours of acute care operation.

[1] For the purposes of NPC 2 and NPC 5, all enumerated items within Table 11.1 shall meet the requirements of Section 1632A of 2001 California Building Code (CBC) or equivalent provision in later version of the CBC by the specified timeframe as indicated by their respective NPC.

[2] For the purposes of NPC 3 and NPC 4, all enumerated items within Table 11.1 shall meet the requirements of the 1998 CBC, Section 1630B, by the specified timeframe. For the purposes of NPC 3R, all enumerated items within Table 11.1 shall meet the requirements of the 1995 CBC, Section 1630A, using I_p=1.0, by the specified timeframe.
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NOTATION:
- Authority: Health and Safety Code Section 130005(g) & 130021
- Reference: Health and Safety Code Section 1275, 129850 & 130005(g)

CHAPTER 7. SAFETY STANDARDS FOR HEALTH FACILITIES

Article 1.0 General.

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Article 2. Definitions.

Unless otherwise stated, the words and phrases defined in this article shall have the meaning stated therein throughout Chapter 7, Part 1, Title 24.

7-111. Definitions.

"Addition" means any work which increases the floor or roof area or the volume of enclosed space of an existing building. ~~and is dependent on the structural elements of that facility for vertical or lateral support.~~

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~~**"Maximum probable earthquake"** means the maximum probable earthquake-induced ground motion having a 10 percent probability of being exceeded in 50 years.~~

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"Office" means the Facilities Development Division within the Office of Statewide Health Planning and Development.

"Primary Gravity Load Resisting System (PGLRS)" means assembly of structural elements in the building that resists gravity loads, including floor and roof beams / girders supporting gravity loads or any other members designed to support significant gravity loads. Foundations supporting loads from the PGLRS shall be considered part of the PGLRS.

"Reconstruction" means the rebuilding of any "existing building" to bring it into full compliance with these regulations and all applicable parts of the California Building Standards Code.

"Seismic Force Resisting System (SFRS)" means assembly of structural elements in the building that resists seismic loads, including struts, collectors, chords, diaphragms and trusses. Foundations supporting loads from the SFRS shall be considered part of the SFRS.

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"Structural repairs" means any change affecting existing or requiring new structural elements primarily intended to correct the effects of deterioration or impending or actual failure, regardless of cause.

~~**"Upper bound earthquake"** means the ground motion having a 10 percent probability of being exceeded in a 100-year period or maximum level of motion which may be expected at the building site within the known geological framework.~~

ARTICLE 3. Approval of Plans And Specifications

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7-117. Site Data.
(a) The site data reports shall be required for all proposed construction except:

1. As provided in the Part 2, Title 24.

2. One-story, wood-frame or light steel frame buildings of Type II or V construction and 4,000 square feet or less in floor area not located within Earthquake Fault Zones or Seismic Hazard Zones as shown in the most recently published maps from the California Geological Survey (CGS).

3. Nonstructural alterations.

4. Structural repairs for other than earthquake damage.

5. Incidental structural additions or alterations.

(b) Three copies of site data reports shall be furnished to the Office for review and evaluation prior to the submittal of the project documents for final plan review. Site data reports shall comply with the requirements of these regulations and Part 2, Title 24. Upon the determination that the investigation of the site and the reporting of the findings was adequate for the design of the project, the Office will issue a letter stating the site data reports are acceptable.

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7-125. Final Review of Plans and Specifications.

(a) One copy of the final plans and specifications and site data reports shall be submitted to the Office.

1. Two copies must be submitted if additions, structural alterations or new buildings are included.

2. The plans and specifications shall include: architectural, mechanical, electrical, structural seismic restraint, and fire and life safety details.

(b) Plans and specifications are to be completely and thoroughly checked by the responsible architect or engineer before submission to the Office. Plans and specifications which are incomplete or incorrect will be returned to the applicant.

1. Where a portion of the construction cannot be fully detailed on the approved plans because of variations in product design and manufacture, the approval of the plans for such portion may be deferred until the material suppliers are selected provided the following conditions are met:

A. The plans clearly indicate that a deferred approval by the Office is required for the indicated portions of the work prior to fabrication and installation.

B. The plans and specifications fully describe the performance and loading criteria for such work.

C. The deferred approval submittals are made in conformance with Section 7-153.

Exception: Seismic Force Resisting System (SFRS), Primary Gravity Load Resisting System (PGLRS), and stairs shall not be deferred.

2. Due to the difficulty of anticipating every unsatisfactory condition that might exist in connection with the existing work where alteration or reconstruction work is proposed, the following clause or one of similar meaning shall be included in all specifications to which the Office gives approval in connection with either reconstruction or alteration work: "The intent of the plans and specifications is to reconstruct the hospital building in accordance with the California Building Standards Code, Titles 19 and 24, California Code of Regulations. Should any conditions develop not covered by the approved plans and specifications wherein the finished work will not comply with Title 24,

California Code of Regulations, a change order detailing and specifying the required work shall be submitted to and approved by the Office before proceeding with the work."
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Article 20. Repair of Earthquake Damage

7-300. Plan Review and Approval.

(a) All repair projects are subject to prior plan review, plan approval and construction permit by the Office except as noted in subsection (b).

(b) For emergency repairs carried out without the Office plan review and permit the aftermath of an earthquake, an application for plan review must be submitted with construction documents, fees and letter of transmittal stating the reasons for emergency repairs. Photographs, if available, and reports of damage and repairs should also be submitted with the application. Additional repairs may be required if the emergency repairs do not comply with the code. For alternate fee payment methodology, see Section 129787 of the Health and Safety Code.

(c) Plan reviews for earthquake damage repairs will be performed on a priority basis. The application for plan review should clearly state that the scope of the project is to repair the damage from the earthquake. Where possible, reviews will be made over the counter.

(d) Plan review fees shall be payable for all damage repair projects per the following:

1. 1.64 percent of estimated construction costs for hospitals.

2. 1.50 percent of estimated construction costs for skilled nursing facilities (SNF) or intermediate care facilities (ICF).

3. For alternate fee payment methodology, see Section 129787 of the Health and Safety Code.

4. An examination fee where review of existing plans is required. The fee will be calculated on a time and material basis at the prevailing hourly rates applicable for the review personnel.

7-301. Appeals. The Hospital Building Safety Board shall act as a board of appeals with regard to disagreements between the Office and hospital/SNF/ICF authorities on interpreting the repair policy or the establishment of the degree of damage. (Section 7-159 of Administrative Regulation for the Office)

7-302. Pre-1973 Structures.

These hospital buildings were approved for construction by local building departments prior to March 7, 1973.

(a) All structural repairs shall be made to conform to vertical load requirements of the California Building Code (CBC).

(b) Where lateral load resisting capacity of the building at any level is reduced by 5 percent or less due to earthquake damage, the repairs may be made with the same construction as before, subject to structural detailing requirements of the CBC.

(c) Where lateral load resisting capacity of the building at any level is reduced by more than 5 percent but not more than 10 percent due to earthquake damage, the repairs shall be made in accordance with Section ~~1635B.3.2.2~~ 3411A.3.2.2 of the CBC. ~~The repaired/reconstructed structural elements shall meet structural requirements using an importance factor of I = 1.0. The building after repairs shall be in reasonable compliance with the CBC using an importance factor, I, equal to 0.75.~~

(d) Where lateral load resisting capacity of the building at any level is reduced by more than 10 percent due to earthquake damage, the repairs shall be made such that the primary structural system and the seismic bracing of other components and systems shall conform to the requirements of Section ~~1635B.3.2.3~~ 3411A.3.2.3 of the CBC.

(e) Where earthquake repairs consist of alterations which involve removal of one or more entire stories, permission for repairs will be granted if lateral load resisting capacity of the remaining structure is not reduced. (Section ~~1635B.3.3~~ 3411A.3.2.4, CBC)

(f) Repair/reconstruction of structures should comply with the design and detailing requirements of engineering materials stated in Chapters ~~19~~ 19A, 20, ~~21~~ 21A, 22A and 23, as applicable, and applicable fire-resistive requirements of the CBC.

(g) Epoxy injection repairs shall require submittal of backup information per section 104.11, Appendix Chapter 1 of the CBC. ~~Chapter 16B, Section 1603B.7.~~

(h) Repair of damage to seismic anchorage of equipment and nonstructural items shall comply with Section 3403A.2.3 ~~1644B.13.1~~ of the CBC.

7-303. Post-1973 Structures.

These hospital structures were approved for construction by the Office of the State Architect or Office after March 7, 1973. They are also referred to in the regulations as approved existing buildings.

(a) Repairs to the damage shall be made to restore the load carrying capacities of the affected elements per Section 3411A.3.1 ~~1635B.3.1~~ of the CBC.

(b) Repair of damage to seismic anchorage of equipment and nonstructural items shall comply with Section 3403A.2.3 ~~1630B~~ of the CBC.

7-304. Type V Single Story SNF or ICF.

(a) All structural repairs shall be made to conform to vertical load requirements of the CBC.

(b) Repair of damage of seismic anchorage of equipment shall comply with the CBC.

(c) Where damage has reduced the lateral load capacity by more than 10% in any one line of the lateral force resisting system in the building, repairs of structural elements shall conform to Section 3403.2.3 of the CBC.

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NOTATION:

- *Authority: Health and Safety Code Section 130005(g) & 130021*
- *Reference: Health and Safety Code Section 1275, 129850 & 130005(g)*